QUASI-GREEN'S FUNCTION APPROACH TO FUNDAMENTAL FREQUENCY ANALYSIS OF ELASTICALLY SUPPORTED THIN CIRCULAR AND ANNULAR PLATES WITH ELASTIC CONSTRAINTS

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Free vibration analysis of homogeneous and isotropic thin circular and annular plates with discrete elements such as elastic ring supports is considered. The general form of quasi-Green's function for thin circular and annular plates is obtained. The nonlinear characteristic equations are defined for thin circular and annular plates with different boundary conditions and different combinations of the core and support radius. The continuity conditions at the ring supports are omitted based on the properties of Green's function. The fundamental frequency of axisymmetric vibration has been calculated using the Newton-Raphson method and calculation software. The obtained results are compared with selected results presented in literature. The exact frequencies of vibration presented in a non-dimensional form can serve as benchmark values for researchers to validate their numerical methods when applied for uniform thin circular and annular plate problems.

Keywords: quasi-Green's function, ring supports, movable edges, elastic constraints

1. Introduction

The study of vibration of a thin circular and annular plate is basic in structural mechanics. Components of circular and annular plates are commonly used in the aerospace industry and aviation as well as in marine and civil engineering applications. Circular and annular plates are the most critical structural elements in high speed rotating engineering systems. The natural frequencies of circular and annular plates have been studied extensively for more than a century, because if only the frequency of external load matches the natural frequency of the plate, destruction may occur. Additionally, the influence of elastic or rigid ring supports on dynamic behavior of plates have been studied in a lot of works, because it used to stabilize or to increase the frequency of plates. Knowledge about distribution of ring supports of variable stiffness can allow one to predict dynamic behavior of structural elements such us circular and annular plates.

The free vibration of circular and annular plates with concentric ring supports have been studied in a lot of works. Bodine (1967) studied the influence of rigid supports on the fundamental frequency of circular plates in which radius of the supports was small. Kunukkasseril and Swamidas (1974) formulated equations for circular plates with elastic supports, but they solved the free vibration problem for a free circular plate. Singh and Mirza (1976) studied free axisymmetric vibration of circular plates elastically supported along two concentric circles. Azimi (1988) studied natural vibration of circular plates with elastic and rigid supports using the receptance method. Wang and Thevendran (1993) analyzed free vibration of annular plates with concentric supports using by the Rayleigh-Ritz method. Ding (1994) solved the free vibration problem for arbitrarily shaped plates with concentric elastic and rigid ring supports. Liu and Chen (1995) studied axisymmetric vibration of annular and circular plates using simple finite analysis. In works by Vega *et al.* (1999) free vibration analysis was presented for a concentrically

supported annular plate with a free edge using the optimized Rayleigh-Ritz method. Laura *et al.* (1999) analyzed transverse vibration of a circular plate with a free edge and concentric ring supports. Vega *et al.* (2000) analyzed free vibration of concentrically supported annular plates with one edge clamped or simply supported. The fundamental frequency of a free thin circular plate supported on a ring was analyzed by Wang (2001). Influence of the stiffness and location of elastic ring supports on the fundamental frequency of circular plates were analyzed by Wang and Wang (2003). Wang (2006, 2014) studied vibration modes of concentrically supported free circular and annular plates with movable edges. Rao and Rao (2014a) analyzed free vibration of annular plates with both edges elastically restrained and resting on the Winkler foundation. Additionally, Rao and Rao (2014b) analyzed free vibration of a thin circular plate with concentric ring and elastic edge support.

In the works presented above, the analyzed plates were separated into two regions for one ring supports. The number of separated regions increases if the number of considered elastic ring supports increases. In this approach, the solution to boundary value problem is complicated. Additionally, continuity conditions between the support and plate must be used to obtain characteristic equations. Solution to the boundary value problem is very tedious and more complicated based on continuity conditions, because characteristic matrices have a large dimension.

Application of Green's function to the solution to the boundary value problem of free vibration of plates allow one to neglect the continuity condition. In the works of Kukla and Szewczyk (2004, 2005, 2007) Green's function approach to frequency analysis is presented for circular and annular thin plates with elastic supports. The authors calculated nontrivial constants of general solutions to the differential equation to obtain a full form of Green's function for free, simply-supported and clamped plates. The nontrivial constants have a very complicated form, and calculating them is very tedious for different boundary conditions such as sliding supports or elastic constraints.

The novelty of the paper is quasi-Green's function (not full form) approach to obtain characteristic equations of concentrically supported circular and annular plates with clamped, free, simply-supported and sliding (movable) edges or elastic constraints. The quasi-Green function is obtained by the method presented in the previous works (Żur, 2015, 2016a). Nonlinear characteristic equations of plates are obtained without calculating nontrivial constants of the general solution to the differential equation. The numerical results of investigation are compared with selected results presented in literature. The exact fundamental frequencies of axisymmetric vibration are presented in a non-dimensional form for different combinations of the core and support radius as well as selected values of parameters of elastic constraints.

2.   Statement of the problem

Consider an isotropic, homogeneous annular (circular) thin plate of constant thickness h in cylindrical coordinates (r, θ, z) with the z-axis along the longitudinal direction. The geometry and coordinate system of the considered plate is shown in Fig. 1. The partial differential equation for free vibration of thin uniform annular (circular) plates has the following form

$$\nabla^4 W(r,t) + \frac{\rho h}{D}\frac{\partial^2 W(r,t)}{\partial t^2} = -\sum_{j=1}^{\chi} K_j W(r,t)\delta(r - r_j) \tag{2.1}$$

where ρ is mass density, $D = Eh^3/[12(1 - \nu^2)]$ is flexural rigidity, E is Young's modulus, ν is Poisson's ratio, $\nabla^2 = (\partial^2/\partial r^2) + (1/r)(\partial/\partial r)$ is Laplacian, K_j is a coefficient of normalized stiffness of the supports, δ is Dirac's delta function, r_j is the position of elastic ring supports, χ is the number of elastic ring supports and $W(r,t)$ is small deflection compared with thickness h of the plate.
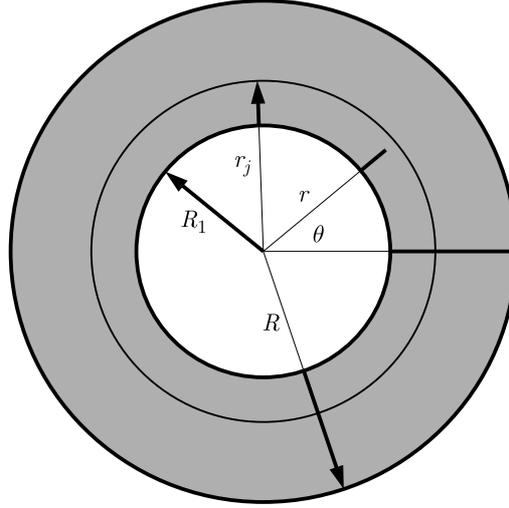
Fig. 1. The geometry and coordinate system of the annular plate with radius of the hole R_1

The axisymmetric deflection of an annular (circular) plate may be expressed as follows

$$W(r,t) = w(r)e^{i\omega t} \tag{2.2}$$

where $w(r)$ is the radial mode function, ω is the natural frequency, and $i^2 = -1$. Substituting Eq. (2.2) into Eq. (2.1) and using the dimensionless coordinates $\xi = r/R$ and $\kappa_j = r_j/R$, the governing differential equation of the annular (circular) plate is obtained

$$L(w) - \lambda^2 w = -\sum_{j=1}^{\chi} K_j w(\kappa_j)\delta(\xi - \kappa_j) \tag{2.3}$$

where

$$L(w) \equiv \frac{d^4 w}{d\xi^4} + \frac{2}{\xi}\frac{d^3 w}{d\xi^3} - \frac{1}{\xi^2}\frac{d^2 w}{d\xi^2} + \frac{1}{\xi^3}\frac{dw}{d\xi} \tag{2.4}$$

is the differential operator and

$$\lambda = \omega R^2 \sqrt{\rho h/D} \tag{2.5}$$

is the dimensionless frequency of vibration.

The boundary conditions at the outer edge ($\xi = 1$) of the annular (circular) plate may be one of the following: clamped, simply supported, free, sliding supports and elastic supports. These conditions may be written in terms of the radial mode function $w(\xi)$ in the following form:
— clamped

$$w(\xi)|_{\xi=1} = 0 \qquad \frac{dw}{d\xi}\Big|_{\xi=1} = 0 \tag{2.6}$$

— simply supported

$$w(\xi)|_{\xi=1} = 0 \qquad M(w)|_{\xi=1} = \left(\frac{d^2 w}{d\xi^2} + \frac{\nu}{\xi}\frac{dw}{d\xi}\right)_{\xi=1} = 0 \tag{2.7}$$

— free

$$M(w)|_{\xi=1} = 0 \qquad V(w)|_{\xi=1} = \left(\frac{d^3 w}{d\xi^3} + \frac{1}{\xi}\frac{d^2 w}{d\xi^2} - \frac{1}{\xi^2}\frac{dw}{d\xi}\right)_{\xi=1} = 0 \tag{2.8}$$

— movable edges (sliding)

$$\left.\frac{dw}{d\xi}\right|_{\xi=1} = 0 \qquad V(w)|_{\xi=1} = 0 \tag{2.9}$$

— elastic supports

$$\begin{aligned}
\Phi(w)|_{\xi=1} &= \left[\left(\frac{d^2w}{d\xi^2} + \nu\frac{dw}{d\xi}\right) + \phi\frac{dw}{d\xi}\right]_{\xi=1} = 0 \\
\Psi(w)|_{\xi=1} &= \left[\left(\frac{d^3w}{d\xi^3} + \frac{d^2w}{d\xi^2} - \frac{dw}{d\xi}\right) - \psi w\right]_{\xi=1} = 0
\end{aligned} \tag{2.10}$$

$M(w)$ and $V(w)$ are the normalized radial bending moment and the normalized effective shear force, respectively. $\phi = K_\phi R/D_R$ and $\psi = K_\psi R^3/D_R$ are the parameters of elastic constraints. K_ϕ and K_ψ are the rotational and translational spring constants, respectively. Similar boundary conditions may be defined at the inner edge ($\xi = R_1/R = \xi_1$), depending on considered annular plates.

3. Finding quasi-Green's function

The general solution to the homogeneous differential equation for thin annular (circular) plates

$$L(w) - \lambda^2 w = 0 \tag{3.1}$$

is a linear combination of the Bessel functions presented in the following form (McLachlan, 1955)

$$w(\xi) = C_1 J_0(\lambda\xi) + C_2 I_0(\lambda\xi) + C_3 Y_0(\lambda\xi) + C_4 K_0(\lambda\xi) \tag{3.2}$$

where $J_0(\lambda\xi)$, $Y_0(\lambda\xi)$ are the Bessel functions of the first and second kind, $I_0(\lambda\xi)$, $K_0(\lambda\xi)$ are the modified Bessel functions of the first and second kind. The quasi-Green function $K(\xi, \alpha)$ is a particular solution to Eq. (3.1) and may be received from the formula presented in the following form (Jaroszewicz and Zoryj, 2005; Żur, 2015)

$$K(\xi, \alpha) = \frac{D(\xi, \alpha)}{W(\alpha)p_0(\alpha)} \tag{3.3}$$

where $p_0(\alpha) = 1$ is a coefficient placed in front of the highest order of derivative of differential equation (3.1), and

$$D(\xi, \alpha) = \begin{vmatrix} J_0(\lambda\alpha) & I_0(\lambda\alpha) & Y_0(\lambda\alpha) & K_0(\lambda\alpha) \\ \dfrac{dJ_0(\lambda\alpha)}{d\alpha} & \dfrac{dI_0(\lambda\alpha)}{d\alpha} & \dfrac{dY_0(\lambda\alpha)}{d\alpha} & \dfrac{dK_0(\lambda\alpha)}{d\alpha} \\ \dfrac{d^2J_0(\lambda\alpha)}{d\alpha^2} & \dfrac{d^2I_0(\lambda\alpha)}{d\alpha^2} & \dfrac{d^2Y_0(\lambda\alpha)}{d\alpha^2} & \dfrac{d^2K_0(\lambda\alpha)}{d\alpha^2} \\ J_0(\lambda\xi) & I_0(\lambda\xi) & Y_0(\lambda\xi) & K_0(\lambda\xi) \end{vmatrix}$$

$$W(\alpha) = \begin{vmatrix} J_0(\lambda\alpha) & I_0(\lambda\alpha) & Y_0(\lambda\alpha) & K_0(\lambda\alpha) \\ \dfrac{dJ_0(\lambda\alpha)}{d\alpha} & \dfrac{dI_0(\lambda\alpha)}{d\alpha} & \dfrac{dY_0(\lambda\alpha)}{d\alpha} & \dfrac{dK_0(\lambda\alpha)}{d\alpha} \\ \dfrac{d^2J_0(\lambda\alpha)}{d\alpha^2} & \dfrac{d^2I_0(\lambda\alpha)}{d\alpha^2} & \dfrac{d^2Y_0(\lambda\alpha)}{d\alpha^2} & \dfrac{d^2K_0(\lambda\alpha)}{d\alpha^2} \\ \dfrac{d^3J_0(\lambda\alpha)}{d\alpha^3} & \dfrac{d^3I_0(\lambda\alpha)}{d\alpha^3} & \dfrac{d^3Y_0(\lambda\alpha)}{d\alpha^3} & \dfrac{d^3K_0(\lambda\alpha)}{d\alpha^3} \end{vmatrix} \tag{3.4}$$

The elements of the matrix $\mathbf{D}$ and $\mathbf{W}$ have the following form

$$\frac{dJ_0(\lambda\alpha)}{d\alpha} = -\lambda J_1(\lambda\alpha) \qquad \frac{dI_0(\lambda\alpha)}{d\alpha} = \lambda I_1(\lambda\alpha)$$

$$\frac{dY_0(\lambda\alpha)}{d\alpha} = -\lambda Y_1(\lambda\alpha) \qquad \frac{dK_0(\lambda\alpha)}{d\alpha} = -\lambda K_1(\lambda\alpha) \tag{3.5}$$

$$\frac{d^2 J_0(\lambda\alpha)}{d\alpha^2} = \frac{\lambda^2}{2}[J_0(\lambda\alpha) + J_2(\lambda\alpha)] \qquad \frac{d^2 I_0(\lambda\alpha)}{d\alpha^2} = \frac{\lambda^2}{2}[I_0(\lambda\alpha) + I_2(\lambda\alpha)]$$

$$\frac{d^2 Y_0(\lambda\alpha)}{d\alpha^2} = \frac{\lambda^2}{2}[Y_0(\lambda\alpha) + Y_2(\lambda\alpha)] \qquad \frac{d^2 K_0(\lambda\alpha)}{d\alpha^2} = \frac{\lambda^2}{2}[K_0(\lambda\alpha) + K_2(\lambda\alpha)] \tag{3.6}$$

$$\frac{d^3 J_0(\lambda\alpha)}{d\alpha^3} = \frac{\lambda^3}{4}[3J_1(\lambda\alpha) + J_3(\lambda\alpha)] \qquad \frac{d^3 I_0(\lambda\alpha)}{d\alpha^3} = \frac{\lambda^3}{4}[3I_1(\lambda\alpha) + I_3(\lambda\alpha)]$$

$$\frac{d^3 Y_0(\lambda\alpha)}{d\alpha^3} = \frac{\lambda^3}{4}[3Y_1(\lambda\alpha) - Y_3(\lambda\alpha)] \qquad \frac{d^3 K_0(\lambda\alpha)}{d\alpha^3} = -\frac{\lambda^3}{4}[3K_1(\lambda\alpha) + K_3(\lambda\alpha)] \tag{3.7}$$

After calculations, the function $D(\xi, \alpha)$ has the form

$$D(\xi, \alpha) = \frac{2\lambda^2}{\pi\alpha}[2I_0(\lambda\xi)K_0(\lambda\alpha) - 2I_0(\lambda\alpha)K_0(\lambda\xi) + \pi J_0(\lambda\xi)Y_0(\lambda\alpha) - \pi J_0(\lambda\alpha)Y_0(\lambda\xi)] \tag{3.8}$$

Bessel function (3.2) expresses linear independent solutions, thus the Wronskian must satisfy the condition (Stakgold and Holst, 2011)

$$W(\alpha) = \frac{8\lambda^4}{\pi\alpha^2} \neq 0 \tag{3.9}$$

Condition (3.9) is satisfied for a circular plate ($0 < \alpha \leqslant 1$) and an annular plate ($0 < \xi_1 \leqslant \alpha \leqslant 1$). After calculations, the quasi-Green function has the form

$$K(\xi, \alpha) = \frac{\alpha}{4\lambda^2}[2I_0(\lambda\xi)K_0(\lambda\alpha) - 2I_0(\lambda\alpha)K_0(\lambda\xi) - \pi J_0(\lambda\alpha)Y_0(\lambda\xi) + \pi J_0(\lambda\xi)Y_0(\lambda\alpha)] \tag{3.10}$$

and satisfies the conditions

$$K(a, a) = \frac{\partial K(\xi, \alpha)}{\partial\xi}\bigg|_{\xi=a} = \frac{\partial^2 K(\xi, \alpha)}{\partial\xi^2}\bigg|_{\xi=a} = 0 \qquad \frac{\partial^3 K(\xi, \alpha)}{\partial\xi^3}\bigg|_{\xi=a} = 1 \tag{3.11}$$

according to properties of the influence functions (Stakgold and Holst, 2011).

4. Solution of the problem for the circular plate

In the previous paper (Żur, 2016b), the possibility of solving the similar boundary value problem was proposed for non-uniform annular plates without calculations. Based on the paper of Żur (2016b), the limit $\lim_{\xi\to 0} Y_0(\lambda\xi) = \infty$, $\lim_{\xi\to 0} K_0(\lambda\xi) = \infty$ of linear independent solutions to Eq. (2.3) for the circular plate can be presented in the following form

$$K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a = J_0(\lambda\xi) - \sum_{j=1}^{\chi} K_j J_0(\lambda\kappa_j)G(\xi, \kappa_j)$$

$$K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b = I_0(\lambda\xi) - \sum_{j=1}^{\chi} K_j I_0(\lambda\kappa_j)G(\xi, \kappa_j) \tag{4.1}$$

where

$$G(\xi, \kappa_j) = K(\xi, \kappa_j)H(\xi - \kappa_j)$$
$$K(\xi, \kappa_j) = \frac{\kappa_j}{4\lambda^2}[2I_0(\lambda\xi)K_0(\lambda\kappa_j) - 2I_0(\lambda\kappa_j)K_0(\lambda\xi) - \pi J_0(\lambda\kappa_j)Y_0(\lambda\xi) + \pi J_0(\lambda\xi)Y_0(\lambda\kappa_j)] \tag{4.2}$$

and

$$\boldsymbol{\kappa} = [\kappa_1, \ldots, \kappa_\chi] \qquad \mathbf{K} = [K_1, \ldots, K_\chi] \tag{4.3}$$

and $H(\xi - \kappa_j)$ is the Heaviside function.

The characteristic equations $\Delta = 0$ of the circular plate for different boundary conditions and different values of the parameters κ_j and K_j are obtained from well known characteristic determinants given by:

— clamped

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a & K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b \\ \dfrac{\partial K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a}{\partial\xi} & \dfrac{\partial K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b}{\partial\xi} \end{vmatrix} |_{\xi=1} \tag{4.4}$$

— simply supported

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a & K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b \\ M[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a] & M[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b] \end{vmatrix}_{\xi=1} \tag{4.5}$$

— free

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} M[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a] & M[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b] \\ V[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a] & V[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b] \end{vmatrix}_{\xi=1} \tag{4.6}$$

— sliding

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} \dfrac{\partial K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a}{\partial\xi} & \dfrac{\partial K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b}{\partial\xi} \\ V[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a] & V[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b] \end{vmatrix}_{\xi=1} \tag{4.7}$$

— elastic supports

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}, \phi, \psi) \equiv \begin{vmatrix} \Phi[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a] & \Phi[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b] \\ \Psi[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a] & \Psi[K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b] \end{vmatrix}_{\xi=1} \tag{4.8}$$

5. Solution of the problem for the annular plate

The linear independent solutions to Eq. (2.3) for the annular plate can be presented in the following form

$$\mathcal{B}_a \equiv K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_a = J_0(\lambda\xi) - \sum_{j=1}^{\chi} K_j J_0(\lambda\kappa_j)G(\xi, \kappa_j)$$

$$\mathcal{B}_b \equiv K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_b = I_0(\lambda\xi) - \sum_{j=1}^{\chi} K_j I_0(\lambda\kappa_j)G(\xi, \kappa_j)$$

$$\mathcal{B}_c \equiv K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_c = Y_0(\lambda\xi) - \sum_{j=1}^{\chi} K_j Y_0(\lambda\kappa_j)G(\xi, \kappa_j) \tag{5.1}$$

$$\mathcal{B}_d \equiv K(\xi, \lambda, \boldsymbol{\kappa}, \mathbf{K})_d = K_0(\lambda\xi) - \sum_{j=1}^{\chi} K_j K_0(\lambda\kappa_j)G(\xi, \kappa_j)$$

The characteristic equations $\Delta = 0$ of the annular plate for different boundary conditions and different values of the parameters κ_j and K_j are obtained from well known characteristic determinants given by:

— free outer edge and clamped inner edge

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} M[\mathcal{B}_a]|_{\xi=1} & M[\mathcal{B}_b]|_{\xi=1} & M[\mathcal{B}_c]|_{\xi=1} & M[\mathcal{B}_d]|_{\xi=1} \\ V[\mathcal{B}_a]|_{\xi=1} & V[\mathcal{B}_b]|_{\xi=1} & V[\mathcal{B}_c]|_{\xi=1} & V[\mathcal{B}_d]|_{\xi=1} \\ \mathcal{B}_a|_{\xi=\xi_1} & \mathcal{B}_b|_{\xi=\xi_1} & \mathcal{B}_c|_{\xi=\xi_1} & \mathcal{B}_d|_{\xi=\xi_1} \\ \left.\dfrac{\partial \mathcal{B}_a}{\partial \xi}\right|_{\xi=\xi_1} & \left.\dfrac{\partial \mathcal{B}_b}{\partial \xi}\right|_{\xi=\xi_1} & \left.\dfrac{\partial \mathcal{B}_c}{\partial \xi}\right|_{\xi=\xi_1} & \left.\dfrac{\partial \mathcal{B}_d}{\partial \xi}\right|_{\xi=\xi_1} \end{vmatrix} \tag{5.2}$$

— free outer edge and simply supported inner edge

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} (M[\mathcal{B}_a]|_{\xi=1} & M[\mathcal{B}_b]|_{\xi=1} & M[\mathcal{B}_c]|_{\xi=1} & M[\mathcal{B}_d]|_{\xi=1} \\ V[\mathcal{B}_a]|_{\xi=1} & V[\mathcal{B}_b]|_{\xi=1} & V[\mathcal{B}_c]|_{\xi=1} & V[\mathcal{B}_d]|_{\xi=1} \\ \mathcal{B}_a|_{\xi=\xi_1} & \mathcal{B}_b|_{\xi=\xi_1} & \mathcal{B}_c|_{\xi=\xi_1} & \mathcal{B}_d|_{\xi=\xi_1} \\ M[\mathcal{B}_a]|_{\xi=\xi_1} & M[\mathcal{B}_b]|_{\xi=\xi_1} & M[\mathcal{B}_c]|_{\xi=\xi_1} & M[\mathcal{B}_d]|_{\xi=\xi_1} \end{vmatrix} \tag{5.3}$$

— free both edges

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} M[\mathcal{B}_a]|_{\xi=1} & M[\mathcal{B}_b]|_{\xi=1} & M[\mathcal{B}_c]|_{\xi=1} & M[\mathcal{B}_d]|_{\xi=1} \\ V[\mathcal{B}_a]|_{\xi=1} & V[\mathcal{B}_b]|_{\xi=1} & V[\mathcal{B}_c]|_{\xi=1} & V[\mathcal{B}_d]|_{\xi=1} \\ M[\mathcal{B}_a]|_{\xi=\xi_1} & M[\mathcal{B}_b]|_{\xi=\xi_1} & M[\mathcal{B}_c]|_{\xi=\xi_1} & M[\mathcal{B}_d]|_{\xi=\xi_1} \\ V[\mathcal{B}_a]|_{\xi=\xi_1} & V[\mathcal{B}_b]|_{\xi=\xi_1} & V[\mathcal{B}_c]|_{\xi=\xi_1} & V[\mathcal{B}_d]|_{\xi=\xi_1} \end{vmatrix} \tag{5.4}$$

— free outer edge and sliding inner edge

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} M[\mathcal{B}_a]|_{\xi=1} & M[\mathcal{B}_b]|_{\xi=1} & M[\mathcal{B}_c]|_{\xi=1} & M[\mathcal{B}_d]|_{\xi=1} \\ V[\mathcal{B}_a]|_{\xi=1} & V[\mathcal{B}_b]|_{\xi=1} & V[\mathcal{B}_c]|_{\xi=1} & V[\mathcal{B}_d]|_{\xi=1} \\ \left.\dfrac{\partial \mathcal{B}_a}{\partial \xi}\right|_{\xi=\xi_1} & \left.\dfrac{\partial \mathcal{B}_b}{\partial \xi}\right|_{\xi=\xi_1} & \left.\dfrac{\partial \mathcal{B}_c}{\partial \xi}\right|_{\xi=\xi_1} & \left.\dfrac{\partial \mathcal{B}_d}{\partial \xi}\right|_{\xi=\xi_1} \\ V[\mathcal{B}_a]|_{\xi=\xi_1} & V[\mathcal{B}_b]|_{\xi=\xi_1} & V[\mathcal{B}_c]|_{\xi=\xi_1} & V[\mathcal{B}_d]|_{\xi=\xi_1} \end{vmatrix} \tag{5.5}$$

— free inner edge and clamped outer edge

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}) \equiv \begin{vmatrix} M[\mathcal{B}_a]|_{\xi=\xi_1} & M[\mathcal{B}_b]|_{\xi=\xi_1} & M[\mathcal{B}_c]|_{\xi=\xi_1} & M[\mathcal{B}_d]|_{\xi=\xi_1} \\ V[\mathcal{B}_a]|_{\xi=\xi_1} & V[\mathcal{B}_b]|_{\xi=\xi_1} & V[\mathcal{B}_c]|_{\xi=\xi_1} & V[\mathcal{B}_d]|_{\xi=\xi_1} \\ \mathcal{B}_a|_{\xi=1} & \mathcal{B}_b|_{\xi=1} & \mathcal{B}_c|_{\xi=1} & \mathcal{B}_d|_{\xi=1} \\ \left.\dfrac{\partial \mathcal{B}_a}{\partial \xi}\right|_{\xi=1} & \left.\dfrac{\partial \mathcal{B}_b}{\partial \xi}\right|_{\xi=1} & \left.\dfrac{\partial \mathcal{B}_c}{\partial \xi}\right|_{\xi=1} & \left.\dfrac{\partial \mathcal{B}_d}{\partial \xi}\right|_{\xi=1} \end{vmatrix} \tag{5.6}$$

— elastic constraints at the inner edge and free outer edge

$$\Delta(\lambda, \boldsymbol{\kappa}, \mathbf{K}, \phi, \psi) \equiv \begin{vmatrix} M[\mathcal{B}_a]|_{\xi=1} & M[\mathcal{B}_b]|_{\xi=1} & M[\mathcal{B}_c]|_{\xi=1} & M[\mathcal{B}_d]|_{\xi=1} \\ V[\mathcal{B}_a]|_{\xi=1} & V[\mathcal{B}_b]|_{\xi=1} & V[\mathcal{B}_c]|_{\xi=1} & V[\mathcal{B}_d]|_{\xi=1} \\ \Phi[\mathcal{B}_a]|_{\xi=\xi_1} & \Phi[\mathcal{B}_b]|_{\xi=\xi_1} & \Phi[\mathcal{B}_c]|_{\xi=\xi_1} & \Phi[\mathcal{B}_d]|_{\xi=\xi_1} \\ \Psi[\mathcal{B}_a]|_{\xi=\xi_1} & \Psi[\mathcal{B}_b]|_{\xi=\xi_1} & \Psi[\mathcal{B}_c]|_{\xi=\xi_1} & \Psi[\mathcal{B}_d]|_{\xi=\xi_1} \end{vmatrix} \tag{5.7}$$

6. Results and discussion

The numerical results for fundamental frequencies of elastically supported circular plates are presented in Tables 1 and 2 with comparison to the results by Azimi (1988), Ding (1994), Wang and Wang. (2003). The numerical results for fundamental frequencies of free vibration of free circular plates with rigid ring supports are presented in Table 3 with comparison to the results by Wang (2014). The numerical results for fundamental frequencies of free vibration of free elastically supported annular plates with different boundary condition at the inner edge are presented in Tables 4 and 5 for different combinations of the radius of the core and supports. The fundamental frequencies of free vibration of circular plates with elastic constraints and interior ring supports of variable stiffness are presented in Table 6. Additionally, the eigenvalues of circular plates with elastic constraints depending on radius and stiffness of interior ring supports are shown in Figs. 2 and 3.

Table 1. The fundamental frequency λ_0 of free vibration of circular plates with the elastic ring support

K_1	κ_1	Dimensionless frequency λ_0	Boundary conditions			
			Clamped	Simply supported	Free	Sliding
10	0	GF	3.196	2.221	0.211	0.212
	0.1	GF	3.272	2.360	1.115	1.171
	0.2	GF	3.326	2.460	1.357	1.383
		Wang and Wang (2003)	3.325	2.460	–	–
		Azimi (1988)	3.326	2.461	–	–
		Ding (1994)	3.322	–	–	–
	0.3	GF	3.348	2.523	1.497	1.532
	0.4	GF	3.338	2.547	1.620	1.656
		Wang and Wang (2003)	3.338	2.547	–	–
		Azimi (1988)	3.338	2.547	–	–
		Ding (1994)	3.334	–	–	–
	0.5	GF	3.304	2.530	1.736	1.765
	0.6	GF	3.262	2.478	1.844	1.856
		Wang and Wang (2003)	3.262	2.478	–	–
		Azimi (1988)	3.262	2.479	–	–
		Ding (1994)	3.262	–	–	–
	0.7	GF	3.225	2.403	1.928	1.928
	0.8	GF	3.204	2.321	1.960	1.980
		Wang and Wang (2003)	3.204	2.321	1.961	–
		Azimi (1988)	3.199	2.321	–	–
		Ding (1994)	3.204	–	–	–

The fundamental frequencies of free vibration of annular plates with the clamped outer edge and the free inner edge (rigid interior support) are presented in Table 7 with comparison to the results by Vega (2000). The eigenvalues of free annular plates with elastic constraints at the inner edge and interior ring supports are presented in Table 8 for different combinations of the radius of the core and supports.

Table 2. The fundamental frequency λ_0 of free vibration of circular plates with the elastic ring support

K_1	κ_1	Dimensionless frequency λ_0	Boundary conditions			
			Clamped	Simply supported	Free	Sliding
1000	0	GF	3.204	2.223	0.666	0.667
	0.1	GF	4.677	3.805	1.946	2.238
	0.2	GF	5.175	4.202	2.049	2.418
		Wang and Wang (2003)	5.175	4.202	–	–
		Azimi (1988)	5.187	4.210	–	–
		Ding (1994)	4.929	–	–	–
	0.3	GF	5.763	4.682	2.187	2.656
	0.4	GF	6.110	5.276	2.374	2.979
		Wang and Wang (2003)	6.110	5.276	–	–
		Azimi (1988)	6.129	5.282	–	–
		Ding (1994)	6.114	–	–	–
	0.5	GF	5.195	5.136	2.619	3.403
	0.6	GF	4.503	4.479	2.891	3.803
		Wang and Wang (2003)	4.503	4.479	–	–
		Azimi (1988)	4.512	4.486	–	–
		Ding (1994)	4.492	–	–	–
	0.7	GF	3.967	3.962	2.992	3.707
	0.8	GF	3.539	3.532	2.787	3.438
		Wang and Wang (2003)	3.539	3.532	–	–
		Azimi (1988)	3.547	3.537	–	–
		Ding (1994)	3.547	–	–	–

Table 3. The fundamental frequency λ_0 of free vibration of free circular plates with the rigid ring support

K_1	κ_1	Dimensionless frequency λ_0	
		GF	Wang (2014)
∞	0	3.751	3.752
	0.1	3.909	3.909
	0.2	4.275	4.275
	0.3	4.851	4.851
	0.4	5.706	5.707
	0.5	6.929	6.929
	0.6	8.396	8.390
	0.7	8.960	8.959
	0.8	7.809	7.809
	0.9	6.235	6.235
	1.0	4.935	4.935

Table 4. The fundamental frequency λ_0 of free vibration of free annular plates with different boundary conditions at the inner edge and interior elastic support

K_1	ξ_1	κ_1	Dimensionless frequency	Boundary conditions at the inner edge			
				Clamped	Simply supported	Free	Sliding
10	0.1	0.2	λ_0	2.043	1.826	1.350	1.364
	0.1	0.4		2.050	1.886	1.617	1.624
	0.1	0.6		2.207	2.091	1.848	1.848
	0.1	0.9		2.658	2.537	1.931	1.946
	0.3	0.5		2.569	1.918	1.730	1.778
	0.3	0.7		2.697	2.209	1.975	1.975
	0.3	0.9		3.003	2.587	1.959	2.041
	0.5	0.7		3.616	2.235	2.039	2.068
	0.5	0.9		3.812	2.736	2.086	2.193
	0.7	0.8		6.077	2.634	2.311	2.363
	0.7	0.9		6.119	3.008	2.401	2.436
	0.8	0.9		9.197	3.275	2.658	2.658

Table 5. The fundamental frequency λ_0 of free vibration of free annular plates with different boundary conditions at the inner edge and interior elastic support

K_1	ξ_1	κ_1	Dimensionless frequency	Boundary conditions at the inner edge			
				Clamped	Simply supported	Free	Sliding
1000	0.1	0.2	λ_0	2.087	1.925	1.979	2.091
	0.1	0.4		2.117	1.105	2.331	2.400
	0.1	0.6		9.335	3.843	2.852	2.934
	0.1	0.9		4.685	4.215	2.467	2.545
	0.3	0.5		2.684	7.401	2.352	2.859
	0.3	0.7		12.106	4.769	2.887	3.561
	0.3	0.9		6.175	5.214	2.439	2.987
	0.5	0.7		13.526	13.323	2.859	4.266
	0.5	0.9		9.260	7.249	2.648	4.106
	0.7	0.8		5.526	6.437	3.293	6.321
	0.7	0.9		8.396	8.054	3.432	7.049
	0.8	0.9		9.697	7.774	4.294	8.334

Fig. 2. The fundamental frequency of the circular plate with elastic constraints ($\phi = 100$, $\psi = 10$) depending on the radius and stiffness of interior ring supports

Fig. 3. The fundamental frequency of the circular plate with elastic constraints ($\phi = 0.1$, $\psi = 100$) depending on the radius and stiffness of interior ring supports

Table 7. The fundamental frequency λ_0 of free vibration of annular plates with the clamped outer edge and free inner edge and the rigid interior support ($K_1 \to \infty$)

ξ_1		κ_1							
		0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9
		Dimensionless frequency λ_0							
0.1	GF	5.335	5.946	6.262	5.537	4.782	4.215	3.786	3.453
	Vega *et al.* (2000)	5.335	5.946	6.262	5.537	4.782	4.215	3.786	3.453
0.2	GF		5.890	6.530	5.996	5.051	4.370	3.880	3.512
	Vega *et al.* (2000)		5.890	6.530	5.996	5.051	4.370	3.880	3.512
0.3	GF			6.723	7.100	5.853	4.856	4.194	3.727
	Vega *et al.* (2000)			6.723	7.100	5.853	4.856	4.194	3.727
0.4	GF				7.912	7.527	5.856	4.821	4.154
	Vega *et al.* (2000)				7.912	7.527	5.856	4.821	4.154
0.5	GF					9.612	7.874	5.969	4.894
	Vega *et al.* (2000)					9.612	7.874	5.969	4.894
0.6	GF						11.912	8.256	6.195
	Vega *et al.* (2000)						11.912	8.256	6.196
0.7	GF							14.146	8.785
	Vega *et al.* (2000)							14.147	8.785
0.8	GF								15.991

Table 6. The fundamental frequency λ_0 of free vibration of circular plates with elastic constraints and the interior ring support

K_1	κ_1	Dimensionless frequency	Elastic parameters at the outer edge	
			$\phi = 100,\ \psi = 10$	$\phi = 0.1,\ \psi = 100$
10	0	λ_0	2.056	2.203
	0.1		2.129	2.337
	0.2		2.190	2.434
	0.3		2.241	2.497
	0.4		2.285	2.524
	0.5		2.320	2.513
	0.6		2.344	2.469
	0.7		2.357	2.400
	0.8		2.361	2.321
100	0	λ_0	2.056	2.203
	0.1		2.441	2.928
	0.2		2.610	3.247
	0.3		2.770	3.486
	0.4		2.960	3.650
	0.5		3.188	3.648
	0.6		3.387	3.445
	0.7		3.318	3.145
	0.8		3.142	2.809
1000	0	λ_0	2.056	3.563
	0.1		2.715	3.600
	0.2		2.850	3.878
	0.3		3.032	4.176
	0.4		3.287	4.490
	0.5		3.629	4.700
	0.6		3.907	4.321
	0.7		3.719	3.786
	0.8		3.429	3.263
∞	0	λ_0	2.714	3.624
	0.1		2.769	3.746
	0.2		2.888	3.973
	0.3		3.068	4.250
	0.4		3.327	4.536
	0.5		3.664	4.700
	0.6		3.908	4.410
	0.7		3.743	3.892
	0.8		3.458	3.354

The Poisson ratio is taken as $\nu = 0.3$ for all considered cases. The numerical results are obtained by using the Newton-Raphson method and Mathematica v10 software. The obtained results are in good agreement with the results obtained by other methods presented in literature and can be used to validate the accuracy of other numerical methods as benchmark values.

Table 8. The fundamental frequency λ_0 of free vibration of free annular plates with elastic constraints at the inner edge and interior ring support

K_1	ξ_1	κ_1	Dimensionless frequency	Elastic parameters at the inner edge	
				$\phi = 100, \psi = 10$	$\phi = 0.1, \psi = 100$
10	0.1	0.2	λ_0	1.146	1.556
	0.1	0.4		1.530	1.740
	0.1	0.6		1.814	2.001
	0.1	0.9		2.031	2.439
	0.3	0.5		1.537	1.956
	0.3	0.7		1.839	2.238
	0.3	0.9		1.974	2.625
	0.5	0.7		1.648	2.296
	0.5	0.9		1.894	2.788
	0.7	0.8		1.510	2.680
	0.7	0.9		1.759	3.050
	0.8	0.9		1.603	3.237
1000	0.1	0.2	λ_0	2.101	1.941
	0.1	0.4		2.460	6.152
	0.1	0.6		3.091	3.663
	0.1	0.9		2.855	4.239
	0.3	0.5		2.906	0.852
	0.3	0.7		3.739	4.951
	0.3	0.9		3.212	6.150
	0.5	0.7		4.332	6.707
	0.5	0.9		4.267	7.911
	0.7	0.8		6.374	7.977
	0.7	0.9		7.185	8.053
	0.8	0.9		8.319	8.196
∞	0.1	0.2	λ_0	3.308	1.296
	0.1	0.4		3.687	1.463
	0.1	0.6		3.972	2.000
	0.1	0.9		3.022	1.351
	0.3	0.5		1.175	1.937
	0.3	0.7		2.071	2.120
	0.3	0.9		3.834	1.271
	0.5	0.7		2.995	2.030
	0.5	0.9		2.644	1.335
	0.7	0.8		1.998	0.999
	0.7	0.9		1.683	1.385
	0.8	0.9		1.046	1.293

7. Conclusions

In this paper, the quasi-Green function has been employed to solve natural vibration of elastically supported thin circular and annular plates with different boundary conditions. The advantage of quasi-Green's function is the obtaining of characteristic equations without calculating nontrivial constants in complicated forms. Additionally, the number of supports of circular and annular

plates does not influence the dimension of characteristic matrices, because the continuity conditions can be neglected. In the presented approach, the solution to the boundary value problem is much simpler. The quasi-Green function approach can be used to the frequency analysis of plates and beams with other discrete elements such as an additional mass or a mass on the spring. The exact frequencies of vibration presented in a non-dimensional form can serve as benchmark values for researchers to validate their numerical methods applied in similar problems presented in the paper.

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